GENERAL NOTES:
1. CONTRACTOR SHALL BE RESPONSIBLE FOR SHORING AND BRACING ALL WORK DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL OSHA REGULATIONS ON THE PROJECT SITE. CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS SHOWN AND NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.

2. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES OF CONSTRUCTION SELECTED BY THE CONTRACTOR OR THE SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL TO THE WORK OF THE CONTRACTOR. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE FAILURE OF THE CONTRACTOR TO PERFORM THE CONSTRUCTION WORK IN ACCORDANCE WITH DRAWINGS. THE COST OF ANY TESTS OR WORK REQUIRED BECAUSE OF CONTRACTOR'S FAILURE TO PERFORM IN ACCORDANCE WITH THE DRAWINGS SHALL BE BORNE BY THE CONTRACTOR.

3. CONTRACTOR SHALL REFER TO OTHER DISCIPLINE'S DRAWINGS AND VISIT SITE TO OBSERVE (E) CONSTRUCTION AND AS-BUILT CONDITIONS. SURVEY PROJECT SITE TO LOCATE UNDERGROUND ITEMS & UTILITIES. REMOVE / RELOCATE EXISTING ITEMS IF REQUIRED FOR NEW CONSTRUCTION. COORDINATE ANY DISRUPTION OF SERVICES WITH OWNER.

4. CONTRACTOR SHALL REFER TO ARCHITECTURAL AND SITE PLAN DRAWINGS TO COORDINATE ALL DIMENSIONS AND ELEVATIONS RELATED TO WORK SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE ALL DIMENSIONS WITH THE FABRICATOR. NOTIFY THE ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.

5. THE CONTRACTOR SHALL COORDINATE ALL ROOF, FLOOR, AND WALL OPENINGS WITH STRUCTURAL, ARCHITECTURAL, AND MECHANICAL DRAWINGS.

6. ALL MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE, CURRENT EDITION.

7. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION, OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION, UNLESS SPECIFICALLY STATED OTHERWISE.

GENERAL NOTES CONTINUED:
8. BOTH BAILEY AND SON ENGINEERING, INC. AND THE ENGINEER WHOSE PROFESSIONAL SEAL IS AFFIXED TO THESE CONTRACT DRAWINGS DISCLAIM ANY IMPLIED WARRANTIES OF ANY KIND WHATSOEVER INCLUDING THE IMPLIED WARRANTY OF MERCHANTABILITY, THE IMPLIED WARRANTY OF FITNESS FOR A PARTICULAR PURPOSE AND ANY IMPLIED WARRANTY OF FITNESS OF THESE DRAWINGS AND/OR SPECIFICATIONS.

9. THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMISSION. DRAWINGS SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING RESPONSIBILITY FOR DIMENSIONS, QUANTITY, QUALITY, AND COORDINATION WITH ALL DISCIPLINES. ELECTRONIC TRANSFER OF CAD FILES TO AID THE CONTRACTOR OR FABRICATOR IS NOT RECOMMENDED BUT AUTOCAD DWG.

10. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION. IN CASE OF DISCREPANCIES, NOTIFY THE ENGINEER FOR INTERPRETATION.

11. THE REHABILITATION OF AN EXISTING STRUCTURE REQUIRES ASSUMPTIONS TO BE MADE REGARDING EXISTING CONDITIONS. THESE ASSUMPTIONS MAY NOT BE VERIFIABLE WITHOUT ADDITIONAL COST OR WITHOUT DESTROYING OTHERWISE SERVICEABLE PORTIONS OF THE STRUCTURE. THE ENGINEER SHALL NOT BE LIABLE FOR ANY COST ARISING FROM THE DISCOVERY OF UNKNOWN CONDITIONS IN THE EXISTING STRUCTURE.

12. THE DETAILER SHALL WORK WITH THE STRUCTURAL AND ARCHITECTURAL DOCUMENTS WHILE PREPARING SHOP DRAWINGS. THE DETAILER SHALL REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. IF THE DETAILER ELECTS TO SCALE THE ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN, THE DETAILER SHALL SUBMIT SHOP DRAWINGS THAT REQUEST ARCHITECTURAL VERIFICATION OF SCALED DIMENSIONS WHEN SUBMITTED FOR APPROVAL.

13. WALL, DOOR, WINDOW LOCATIONS; AND LIMITS OF SLAB ON GRADE EDGES, RECESSED, DEPRESSED AND SLOPED AREAS, AND LIMITS OF ROOF & FLOOR DECK (EDGES & OPENINGS) ARE PRIMARILY THE RESPONSIBILITY OF THE ARCHITECT. CONTRACTOR SHALL ESTABLISH OR DETERMINE SUCH INFORMATION BASED ON ARCHITECTURAL DOCUMENTS PRIOR TO ANY FABRICATION OR CONSTRUCTION OF CONCRETE OR STEEL

DESIGN LOADS AND CRITERIA:
INTERNATIONAL BUILDING CODE, 2018 EDITION

FLOOR DEAD LOAD: 70 psf
FLOOR LIVE LOAD: 40 psf (CLASSROOMS)
50 psf (OFFICES)
80 psf (CORRIDORS ON 2ND AND 3RD FLOOR_
100 psf (AT STAIRS)
100 psf (MEDIA CENTER)

ROOF DEAD LOAD: 20 psf
ROOF LIVE LOAD: 20 psf W/LIVE LOAD REDUCTION AS ALLOWABLE BY CODE

SNOW LOADS:
GROUND SNOW LOAD, Pg = 10 psf
FLAT ROOF SNOW LOAD, Pf = 7.7 psf
EXPOSURE FACTOR, Ce = 1.0
IMPORTANCE FACTOR, Is = 1.1
THERMAL FACTOR, Ct = 1.0

WIND LOAD:
ULTIMATE WIND SPEED (3-SEC. GUST) = 115 MPH
NOMINAL WIND SPEED (3-SEC. GUST) = 89.1 MPH
RISK CATEGORY: III
WIND EXPOSURE: C
COMPONENTS AND CLADDING: PER ASCE 7-10, CHAPTER 30, SECT. 30.7
WITH APPLICABLE ADJUSTMENT FACTORS.
INTERNAL PRESSURE COEFFICIENT: GCpi = ±0.18 <ASCE 7-10, TABLE 26.11-1>

SEISMIC DESIGN DATA:
RISK CATEGORY: III
IMPORTANCE FACTOR, Ie = 1.25
SOIL SITE CLASS: C
MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss = 0.279g, S1 = 0.092g
SPECTRAL RESPONSE COEFFICIENTS: Sds = 0.237g, Sd1 = 0.138g
SEISMIC DESIGN CATEGORY: B

BASIC SEISMIC-FORCE-RESISTING SYSTEM(S):
STRUCTURAL STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
SEISMIC RESPONSE COEFFICIENT(S):
Cs = 0.101
RESPONSE MODIFICATION FACTORS:
R (NORTH/SOUTH) = 3
R (EAST/WEST) = 3
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:
1. FOUNDATIONS ARE DESIGNED FOR 3000 PSF ALLOWABLE SOIL BEARING PRESSURE AND A SOIL SUBGRADE MODULUS (K) OF 140 PCI. CONTRACTOR SHALL VERIFY ADEQUACY OF FOOTING AND SLAB SUBGRADE TO SUPPORT THIS LOADING. EXCAVATE ALL SOIL UNSUITABLE FOR FOUNDATION OR SLAB SUPPORT AS DETERMINED BY A GEOTECHNICAL ENGINEER.

2. FILL UNDER BUILDING SLABS TO BE COMPACTED TO 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698), WITH THE UPPERMOST 12 INCHES COMPACTED TO 98% OF THE SAME SPECIFICATION. MOISTURE CONTENT OF THE FILL, WHILE IT IS BEING COMPACTED, SHALL BE WITHIN 3% OF THE STANDARD PROCTOR OPTIMUM MOISTURE CONTENT.

3. A 6" LAYER OF CRUSHED STONE SHALL BE PLACED BENEATH THE SLAB ON GRADE. THE CRUSHED STONE SHOULD CONSIST OF MACADAM BASE COURSE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY.

4. OWNER MAY RETAIN AN INDEPENDENT GEOTECHNICAL ENGINEER FOR TESTING COMPACTION AND INSPECTIONS OF ALL FOOTING AND SLAB SUBGRADE. TEST AND INSPECTION RESULTS SHALL BE REPORTED IN WRITING TO THE ENGINEER AND CONTRACTOR WITHIN 24 HOURS AFTER TESTS ARE MADE. THE COST OF ANY RETESTS OR ADDITIONAL WORK REQUIRED DUE TO IMPROPERLY COMPACTED FILL SHALL BE BORNE BY THE CONTRACTOR.

5. THE FOUNDATION IS DESIGNED AS RECOMMENDED BY S&ME, INC. REPORT DATED SEPTEMBER 19, 2014. THE ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD CONTRARY TO THOSE ASSUMED FOR DESIGN.

6. SUBGRADE MATERIALS AND THEIR INSTALLATION SHALL BE AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

7. THE CONTRACTOR SHALL RETAIN A COPY OF THE SUBSURFACE REPORT ON PROJECT SITE AND SHALL FOLLOW ALL CONSTRUCTION AND FOUNDATION RECOMMENDATIONS OR PROCEDURES THEREIN. RECOMMENDATIONS MADE IN THE REPORT SHALL BE CONSTRUED AS PROJECT SPECIFICATIONS FOR SITE PREPARATION AND FOUNDATION CONSTRUCTION.

STRUCTURAL STEEL NOTES:
1. DESIGN, FABRICATION, AND ERECTION OF ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AISC MANUAL OF STEEL CONSTRUCTION, 15TH EDITION, UNLESS NOTED OTHERWISE.

2. MATERIALS SHALL MEET THE REQUIREMENTS OF THE FOLLOWING SPECIFICATIONS:

WIDE FLANGE STRUCTURAL STEEL.....ASTM A992, GRADE 50
STRUCTURAL STEEL.....ASTM A36
STRUCTURAL TUBING.....ASTM A500, GRADE B, FY (MIN) = 46 KSI
BOLTS.....ASTM A325-N
WELDING ELECTRODES.....AWS-A5.1, E70XX LOW HYDROGEN (OR EQUAL)
STEEL PIPE.....ASTM A53, TYPE E OR S, GRADE B

3. ALL STRUCTURAL WELDING SHALL BE MADE BY A CERTIFIED WELDER IN ACCORDANCE WITH THE LATEST EDITION OFTHE AWS SPECIFICATIONS D1.1. MINIMUM SIZE OF FILLET WELD SHALL BE 1/16" SMALLER THAN MATERIAL THICKNESS OF THICKER PART JOINED, UNLESS NOTED OTHERWISE. ALL WELDING ELECTRODE STORAGE FOR LOW-HYDROGEN ELECTRODES SHALL BE STORED @ 250° WHEN EXPOSURE EXCEEDS REQUIREMENTS OF COLUMN A, TABLE 51 OF AWS. WELD CLEANING AND PAINTING OF COMPLETED WELDS SHALL BE IN ACCORDANCE WITH AWS.

4. UNLESS NOTED OTHERWISE ON THE PLANS, CONNECTIONS SHALL DEVELOP AT LEAST ONE-HALF OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES OF THE AISC MANUAL FOR THE GIVEN SECTION AND SPAN OF THE BEAM IN QUESTION. IN NO CASE, HOWEVER, SHALL THE LENGTH OF FRAMED CONNECTIONS BE LESS THAN ONE-HALF THE "T" DIMENSION OF THE BEAM WEB. CONNECTIONS INDICATED ON THE PLANS BY "*" SHALL CONTAIN THE MAXIMUM NUMBER OF ROWS OF BOLTS, AT 3" PITCH, THAT CAN BE FIT IN A CLIP ANGLE WHOSE LENGTH EQUALS THE "T" DIMENSION OF THE BEAM.

5. ALL BOLTED CONNECTIONS SHALL BE BEARING-TYPE USING 3/4" DIAMETER AND BROUGHT TO A SNUG TIGHT CONDITION. A325-N BOLTS WITH THREADS INCLUDED IN SHEAR PLANE, UNLESS NOTED OTHERWISE.

6. SHOP CONNECTIONS MAY BE BOLTED OR WELDED.

7. FIELD CONNECTIONS SHALL BE BOLTED UNLESS NOTED OTHERWISE ON DRAWINGS.

8. SURFACE PREPARATIONS FOR STRUCTURAL STEEL SUBJECT TO EXTERIOR ENVIRONMENTAL CONDITIONS SHALL BE CLEANED IN ACCORDANCE SSPC-SP6 (COMMERCIAL BLAST CLEANING) AND PRIMED WITH SSPC PAINT 31. STEEL NOT SUBJECT TO EXTERIOR ENVIRONMENTAL CONDITIONS SHALL BE CLEANED IN ACCORDANCE WITH SSPC - SP2 (HAND TOOL CLEANING). AND PRIMER WITH SSPC PAINT 15, OR BETTER, 2 MILS DFT AND SHALL BE COMPATIBLE WITH OVERCOAT.

LOOSE STEEL LINTELS, ANGLES, LINTEL BOTTOM PLATES, JOISTS AND LINTEL BEARING PLATES, ETC. SHALL BE GALVANIZED. PROPERLY CLEAN SURFACES PRIOR TO WELDING. SEE PROJECT SPECIFICATIONS FOR GALVANIZED MEMBERS THAT MUST BE PRIMED AND PAINTED.

9. PROVIDE MISCELLANEOUS STEEL & SUPPORT ANGLES AROUND COLUMN AND OTHER FLOOR AND ROOF PENETRATIONS AND OPENINGS REQUIRED TO SUPPORT ENDS AND EDGES OF METAL DECK.

10. PROVIDE 3" CONCRETE COVER OVER ALL STEEL BELOW GRADE, EXPOSED TO WEATHER, OR SUBJECT TO MOISTURE.

11. WHEN STRUCTURAL STEEL SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC, CHAPTER 17 APPLY,
A. THE CONTRACTOR SHALL PROVIDE THE STEEL INSPECTOR:
1. WELDER QUALIFICATION CERTIFICATES (DATED WITHIN THE PAST 48 MONTHS)
2. WELD PROCEDURES FOR WHICH WELDERS ARE CERTIFIED.
3. ELECTRODE TYPE TO BE USED FOR STRUCTURAL STEEL.
4. ELECTRODE TYPE TO BE USED FOR METAL DECKING.

B. ALL PJP AND CJP WELDS SHALL BE CONTINUOUSLY MONITORED DURING WELDING.
C. ALL MULTIPASS FILLET WELDS SHALL BE CONTINUOUSLY MONITORED DURING WELDING.
D. SINGLE-PASS FILLET WELDS GREATER THAN 5/16" SHALL BE CONTINUOUSLY MONITORED DURING WELDING.
E. INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE PERIODICALLY INSPECTED DURING INSTALLATION.
F. VERIFICATION OF HIGH STRENGTH BOLTS WILL BE REQUIRED.
G. BEARING-TYPE CONNECTIONS SHALL REQUIRE PERIODIC INSPECTION.

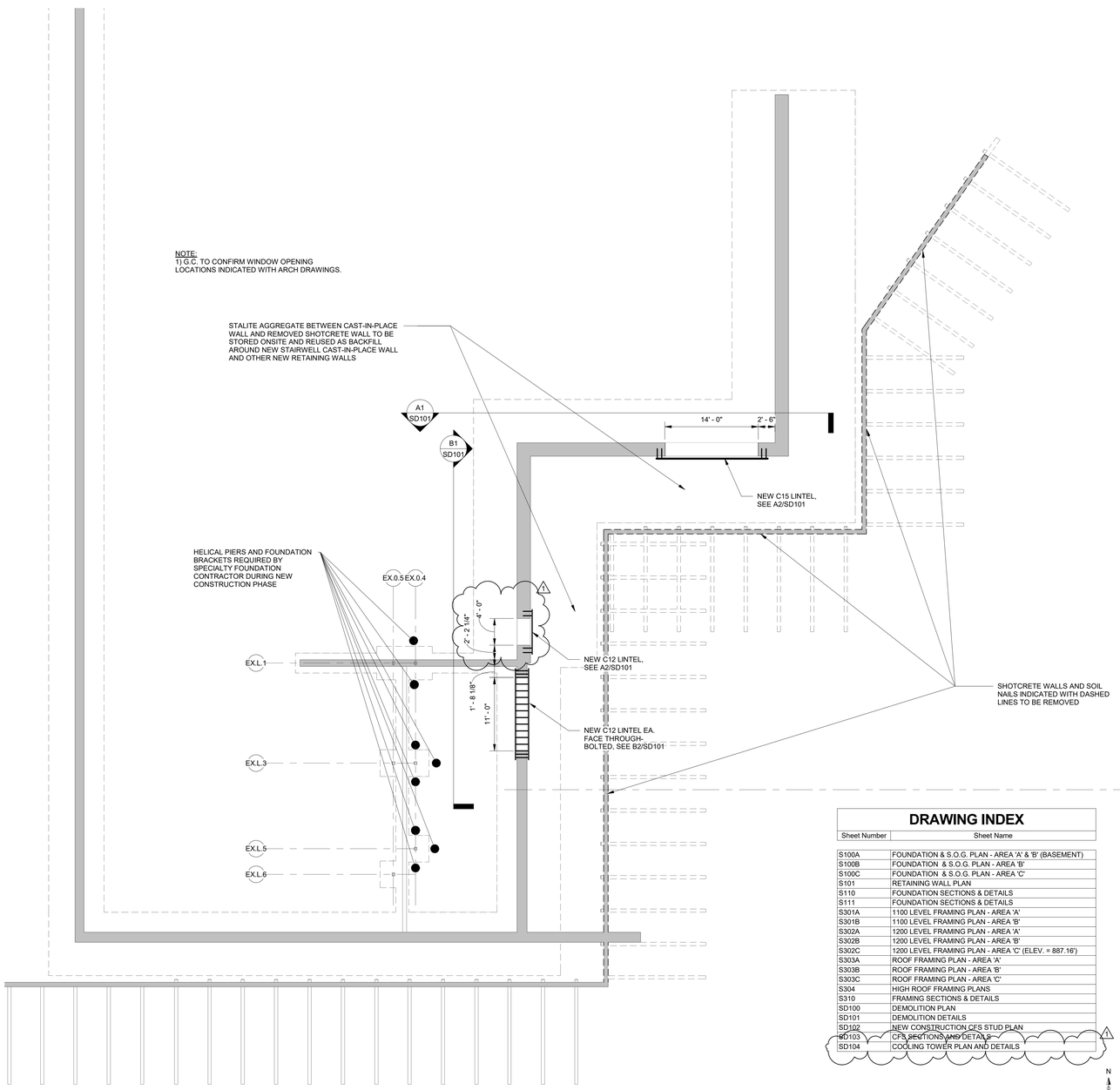
12. PIPING GREATER THAN 4"∅ SHALL BE SUPPORTED @ 10'-0" O.C. MAX. AND SHALL BE CONSIDERED IN THE DESIGN.

POST-INSTALLED REBAR, ANCHORS AND FASTENERS

THE BELOW PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT. PRODUCT DIAMETER AND EMBEDMENT SHALL BE AS SHOWN IN THE DETAILS. INSTALL PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). PRIOR TO INSTALLATION, CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR PRODUCT-SPECIFIC INSTALLATION TRAINING AND A LETTER SHALL BE SUBMITTED TO THE ENGINEER-OF-RECORD (EOR) INDICATING TRAINING HAS TAKEN PLACE. UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS, REFER TO THE PROJECT BUILDING CODE AND/OR EVALUATION REPORT FOR REQUIRED SPECIAL INSPECTIONS AND PROOF LOAD REQUIREMENTS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE EOR FOR REVIEW. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A RESEARCH REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION UNDER THE PROJECT BUILDING CODE. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATIONS THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT.
1. FOR ANCHORING INTO CONCRETE
a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. PRE-APPROVED PRODUCTS INCLUDE:
1. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)
2. SIMPSON STRONG-TIE "TITEN-HD ROD HANGER" (ICC-ES ESR-2713)
b. ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. DESIGN BOND STRENGTH HAS BEEN BASED ON CRACKED CONCRETE, ACI 355.4 TEMPERATURE CATEGORY B, AND INSTALLATIONS INTO DRY HOLES DRILLED USING A HAMMER DRILL INTO CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER WHEN REQUIRED PER ACI 318-11 D.9.2.2. INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACI 318-11 D.9.2.4. PRE-APPROVED PRODUCTS INCLUDE:
1. SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-263)
2. SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) (G.C. OPTION @ 50° F OR WARMER)
3. SIMPSON STRONG-TIE "ET-HP" (ICC-ES ESR-3372) (G.C. OPTION @ 50° F OR WARMER)
c. POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
1. SIMPSON STRONG-TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)
2. SIMPSON STRONG-TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)
2. FOR ANCHORING INTO MASONRY
a. SOLID-GROUTED CONCRETE MASONRY
1. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC106. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)**
2. ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-281)**
2. **SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) (G.C. OPTION @ 50° F OR WARMER)**
3. **SIMPSON STRONG-TIE "ET-HP" (IAPMO-UES ER-241) (G.C. OPTION @ 50° F OR WARMER)**
3. POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)**
2. **SIMPSON STRONG-TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)**
b. HOLLOW CONCRETE MASONRY
1. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "TITEN-HD"**
2. ADHESIVE FOR REBAR AND ANCHORS WITH SCREEN TUBES SHALL HAVE BEEN TESTED FOR USE IN ACCORDANCE WITH ICC-ES AC58. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) (TEMP. ABOVE 50° F)**
3. SPOWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
1. **SIMPSON STRONG-TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)**
2. **SIMPSON STRONG-TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)**
3. FOR FASTENING INTO STEEL: POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
a. SIMPSON STRONG-TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)
b. SIMPSON STRONG-TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)

TYPICAL ABBREVIATIONS:

A.R.....ANCHOR ROD
ACI.....AMERICAN CONCRETE INSTITUTE
AISC.....AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI.....AMERICAN IRON AND STEEL INSTITUTE
APA.....AMERICAN PLYWOOD ASSOCIATION
ARCH.....ARCHITECTURAL
AFPA.....AMERICAN FOREST AND PAPER ASSOCIATION
ASCE.....AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM.....AMERICAN SOCIETY FOR TESTING AND MATERIALS
BaSE,INC....BAILEY & SON ENGINEERING, INC.
BC.....BOTTOM CHORD
B/.....BOTTOM OF
BLKG.....BLOCKING
B.O.D.....BOTTOM OF DECK
BRG.....BEARING
C.J.....CONTROL JOINT
CL.....CENTER LINE
CLR.....CLEAR
CMU.....CONCRETE MASONRY UNIT
CONT.....CONTINUOUS
D.B.E.....DECK BEARING ELEVATION
DBL.....DOUBLE
DIA.....DIAMETER
DL.....DEAD LOAD
(E).....EXISTING
E.W.....EACH WAY
E.O.S.....EDGE OF SLAB
E.N.....EDGE NAIL
FIN.....FINISHED OR FINAL
FIN.FLR.....FINISHED FLOOR
FLR.....FLOOR
Fc.....SPECIFIED CONCRETE STRENGTH @ 28 DAYS
F.O.B.....FACE OF BRICK
F.O.M.....FACE OF MASONRY
F.S.....FAR SIDE
FT.....FOOT OR FEET
GA.....GAUGE
GALV.....GALVANIZED
GLB.....GLU-LAMINATED BEAM
HD.....HOLDOWN
HDR.....HEADER
HORIZ.....HORIZONTAL
HSS.....HOLLOW STRUCTURAL SECTION
IBC.....INTERNATIONAL BUILDING CODE
IN.....INCH OR INCHES
IRC.....INTERNATIONAL RESIDENTIAL CODE
ICBO.....INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS
ICC.....INTERNATIONAL CODE COUNCIL
J.B.E.....JOIST BEARING ELEVATION
K.....KIP
LB.....POUND
LL.....LIVE LOAD
LLH.....LONG LEG HORIZONTAL
LLV.....LONG LEG VERTICAL
LSL.....LAMINATED STRAND LUMBER
LVL.....LAMINATED VENEER LUMBER
LBW.....LOAD BEARING WALL
JT.....JOINT
JST.....JOIST
JH.....JOIST HANGER
MFR.....MANUFACTURER
MAX.....MAXIMUM
MIN.....MINIMUM
ML.....MATCHLINE
(N).....NEW
NTS.....NOT TO SCALE
O.C.....ON CENTER
O.E.....OUTSIDE EDGE
O.H.....OPPOSITE HAND
OSB.....ORIENTED STRAND BOARD
OWT.....OPEN WEB TRUSS
PAF.....POWDER ACTUATED FASTENER
PDF.....POWDER DRIVEN FASTENER
P.E.N.....PANEL EDGE NAIL
P.E.M.B.....PRE-ENGINEERED METAL BUILDING
PF.....PARTIAL FRAME
PL.....PLATE
PLCS.....PLACES
PLF.....POUNDS PER LINEAR FOOT
PSF.....POUNDS PER SQUARE FOOT
PSI.....POUNDS PER SQUARE INCH
PSL.....PARALLEL STRAND LUMBER
PT.....PRESSURE TREATED
RF.....RIGID FRAME
SHTG.....SHEATHING
SJI.....STEEL JOIST INSTITUTE
SDI.....STEEL DECK INSTITUTE
S.O.G.....SLAB ON GRADE
SQ.....SQUARE
STL.....STEEL
T.....TON (2000 LBS.)
T&B.....TOP AND BOTTOM
TC.....TOP CHORD
T/.....TOP OF
THKD.....THICKENED
T.O.C.....TOP OF CONCRETE
TS.....THICKENED SLAB
TYP.....TYPICAL
UNO.....UNLESS NOTED OTHERWISE
V.B.....VAPOR BARRIER
VERT.....VERTICAL
VIF.....VERIFY IN FIELD
VWA.....VERIFY WITH ARCHITECT
WWF.....WELDED WIRE FABRIC
YD.....YARD (3 FT.)
∅.....DIAMETER



DRAWING INDEX

Sheet Number	Sheet Name
S100A	FOUNDATION & S.O.G. PLAN - AREA 'A' & 'B' (BASEMENT)
S100B	FOUNDATION & S.O.G. PLAN - AREA 'B'
S100C	FOUNDATION & S.O.G. PLAN - AREA 'C'
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S110	FOUNDATION SECTIONS & DETAILS
S111	FOUNDATION SECTIONS & DETAILS
S301A	1100 LEVEL FRAMING PLAN - AREA 'A'
S301B	1100 LEVEL FRAMING PLAN - AREA 'B'
S302A	1200 LEVEL FRAMING PLAN - AREA 'A'
S302B	1200 LEVEL FRAMING PLAN - AREA 'B'
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S303A	ROOF FRAMING PLAN - AREA 'A'
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SD100	DEMOLITION PLAN
SD101	DEMOLITION DETAILS
SD102	NEW CONSTRUCTION CFS STUD PLAN
SD103	CFS SECTIONS AND DETAILS
SD104	COOLING TOWER PLAN AND DETAILS

0' 5' 10' 15' 20' 25' 30'
GRAPHIC SCALE

A1 SD100 **1000 LEVEL DEMOLITION PLAN VIEW**
1/8" = 1'-0"

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SEALS

SPARTANBURG COUNTY SCHOOL DISTRICT FIVE
JAMES F. BYRNES HIGH SCHOOL
PHASE 2 ACADEMIC WING ADDITION
150 E. MAIN STREET
DUNCAN, SC 29334

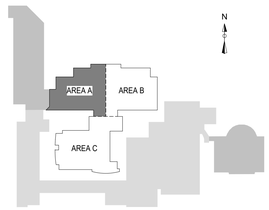
SHEET ISSUE:

NO.	DATE	DESCRIPTION	BY
0	4/21/22	IFC SET	ATR
1	4/29/22	DEMO REVISIONS	BWH

DEMO REVISIONS 4/29/22

PRINCIPAL IN CHARGE: PGG
PROJECT ENGINEER: ATR
DRAWN BY: ATR

SHEET TITLE:
DEMOLITION PLAN



SHEET NO. SD100
PROJ. NO. 20242